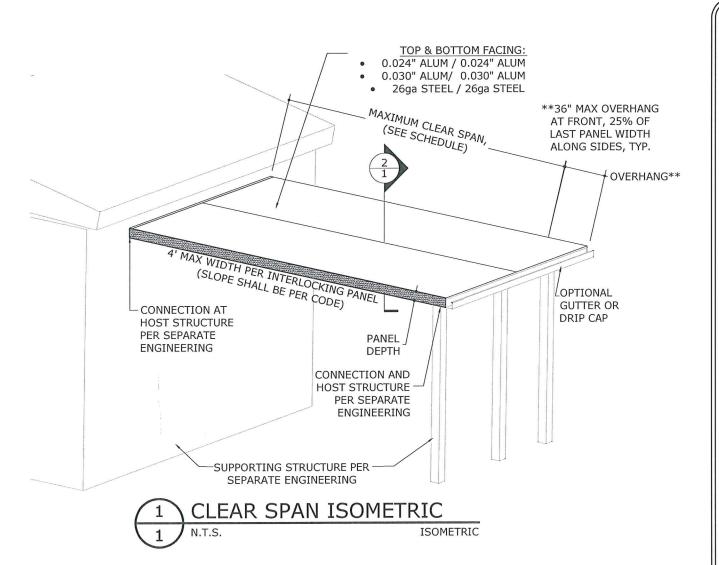
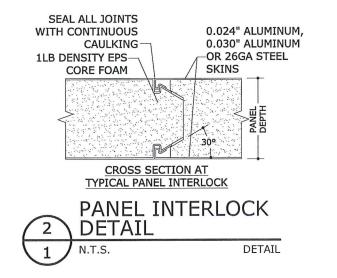
STRUCTALL BUILDING SYSTEMS

EPS FOAM CORE ROOF PANELS - METAL SKIN





MAXIMUM ALLOWABLE **DESIGN PRESSURES:**

AS NOTED IN CLEAR **SPAN TABLE**

DESIGN NOTES:

POSITIVE AND NEGATIVE DESIGN PRESSURES CALCULATED FOR USE WITH THIS SYSTEM SHALL BE DETERMINED BY OTHERS ON A JOB-SPECIFIC BASIS IN ACCORDANCE WITH THE GOVERNING CODE. SITE-SPECIFIC LOAD REQUIREMENTS FOR LIVE LOAD, WIND LOAD, SNOW LOAD OR ANY LOAD COMBINATION SHALL BE DETERMINED IN ACCORDANCE WITH ASCE 7 AND THE 2006, 2009 OR 2012 INTERNATIONAL BUILDING CODE (AS LESS THAN OR EQUAL TO THE POSITIVE OR NEGATIVE DESIGN PRESSURE CAPACITY VALUES LISTED HEREIN FOR ANY ASSEMBLY AS SHOWN.

GENERAL NOTES:

1. THIS SPECIFICATION HAS BEEN DESIGNED AND SHALL BE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF THE 2006, 2009 & 2012 INTERNATIONAL BUILDING CODES, THE 2006, 2009 & 2012 INTERNATIONAL RESIDENTIAL CODE. CONTRACTOR SHALL INVESTIGATE AND CONFORM TO ALL LOCAL BUILDING CODE AMENDMENTS WHICH MAY APPLY. DESIGN CRITERIA BEYOND AS STATED HEREIN MAY REQUIRE ADDITIONAL SITE-SPECIFIC SEALED ENGINEERING. SEISMIC DESIGN HAS NOT

COMPOSITE ROOF PANELS SHALL COMPLY WITH CHAPTER 7 SECTION 719 (IBC 2006 & 2009), CHAPTER 7 SECTION 720 (2012 IBC), CHAPTER 8 SECTION 803, CLASS A INTERIOR FINISH, AND CHAPTER 26 SECTION 2603 OF THE 2006, 2009 & 2012 IBC.

3. NO 33-1/3% INCREASE IN ALLOWABLE STRESS HAS BEEN USED IN THE DESIGN

OF THIS SYSTEM.

4. DESIGN PRESSURES AS NOTED HEREIN ARE BASED ON A MAXIMUM TESTED PRESSURE DIVIDED BY A 2.0 FACTOR OF SAFETY.

THE ARCHITECT/ENGINEER OF RECORD FOR THE PROJECT SUPERSTRUCTURE WITH

WHICH THIS DESIGN IS USED SHALL BE RESPONSIBLE FOR THE INTEGRITY OF ALL SUPPORTING SURFACES TO THIS DESIGN WHICH SHALL BE COORDINATED BY THE PERMITTING CONTRACTOR.

6. SEPARATE 'SITE-SPECIFIC' SEALED ENGINEERING SHALL BE REQUIRED IN ORDER

TO DEVIATE FROM LOADS, DEFLECTIONS, OR SPANS CONTAINED HEREIN. LINEAR INTERPOLATION OF THE ALLOWABLE SPAN TABLES LISTED HEREIN SHALL NOT BE PERMITTED. CONTACT THIS ENGINEER FOR ALTERNATE SPAN CALCULATIONS AS MAY BE

REQUIRED.

7. THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE
INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE
CONDITIONS DETAILED HEREIN, A LICENSED ENGINEER OR REGISTERED ARCHITECT
SHALL PREPARE SITE SPECIFIC DOCUMENTS FOR USE IN CONJUNCTION WITH THIS

8. THE CONTRACTOR SHALL CAREFULLY CONSIDER POSSIBLE IMPOSING LOADS ON ROOF, INCLUDING BUT NOT LIMITED TO ANY CONCENTRATED LOADS WHICH MAY JUSTIFY GREATER DESIGN CRITERIA. THIS ADDITIONAL ROOF LOAD CRITERIA SHALL BE PROPERLY ANALYZED BY A LICENSED ENGINEER OR REGISTERED ARCHITECT.

9. EPS CORE COMPOSITE PANELS SHALL BE CONSTRUCTED USING TYPE 3105-H254
ALUMINUM FACINGS OR ASTM A653, CS, TYPE B HOT DIP GALVANIZED G90 COATED
STEEL FACINGS. EXPANDED POLYSTYRENE FOAM SHALL HAVE TYPICAL DENSITY OF 1.0
PCF. THE EPS FOAM SHALL BE ADHERED TO THE ALUMINUM FACING WITH MORAD M640 SERIES ADHESIVE (BY ROHM AND HAAS COMPANY). FABRICATION SHALL BE IN ACCORDANCE WITH APPROVED FABRICATION METHODS BY MANUFACTURER FOR ALL

10. THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR MATERIALS TO PREVENT ELECTROLYSIS.

11. ENGINEER SEAL AFFIXED HERE TO VALIDATES STRUCTURAL DESIGN AS SHOWN

ONLY, USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION, SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE, & FEDERAL CODES & FROM DEVIATIONS OF THIS PLAN.

12. EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR

AFFIRMATIONS ARE INTENDED. AFFIRMATIONS ARE INTENDED:

13. ALTERATIONS, ADDITIONS, OR OTHER MARKINGS TO THIS DOCUMENT ARE NOT PERMITTED AND INVALIDATE THIS CERTIFICATION.

TABLE VALUE DERIVATIONS:

PANEL PROPERTIES:

1. PANEL STRUCTURAL PROPERTIES DERIVED FROM CERTIFIED TEST REPORTS

Nos. TT-506027B, 506027C, 506027D, 509014A, 509014B BY TERRAPIN

TESTING, ESP012351P-1, ESP012351P-2, ESP012351P-3A, ESP012351P-4, ESP012351P-5, ESP012351P-6, EXP012351P-6A, ESP012351P-7, ESP012351P-8, ESP012351P-9, ESP012351P-9A BY ELEMENT MATERIALS TECHNOLOGY.

PANEL DEAD LOADS HAVE BEEN FACTORED INTO CALCULATIONS FOR LIVE LOADS OR UPLIFT AS WELL AS CALCULATIONS FOR PANEL PROPERTIES.

FRANK L. BENNARDO, P.E. L. BENNAPDO VALID FOR 1 PERMIT ONLY U.N.O. VALID ONLY WITH ORIGINAL ENGINEER SEA

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350 BURBANK ROAD
0LDSMAR, FL 34677
PH: (813) 855-2627
AM CORE ROOF PANELS - M RUCTAL

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SCALE: N.T.S.

MAXIMUM ALLOWABLE CLEAR SPAN TABLE:

		3" Pa	anels		4" Panels			6" Panels	
Live Load &/or Uplift	Deflection Limit (L/)	0.024" Alum Skin	0.030" Alum Skin	0.024" Alum Skin	0.030" Alum Skin	26ga Steel Skin	0.024" Alum Skin	0.030" Alum Skin	26ga Steel Skin
	(0)	1-LB EPS	1-LB EPS	1-LB EPS	1-LB EPS	1-LB EPS	1-LB EPS	1-LB EPS	1-LB EPS
+/- 10 psf	120	14'-10"	16'-0"	17'-6"	19'-9"	20'-3"	21'-9"	24'-0"	23'-0"
+/- 10 psf	180	12'-12"	16'-0"	15'-4"	17'-3"	18'-6"	19'-0"	21'-4"	22'-10"
+/- 10 psf	240	11'-9"	15'-9"	13'-11"	15'-8"	16'-10"	17'-3"	19'-5"	20'-9"
+/- 15 psf	120	12'-12"	16'-0"	15'-4"	17'-3"	16'-6"	18'-1"	20'-2"	20'-5"
+/- 15 psf	180	11'-4"	15'-1"	13'-5"	15'-1"	16'-2"	16'-7"	18'-8"	19'-11"
+/- 15 psf	240	10'-4"	13'-9"	12'-2"	13'-9"	14'-8"	15'-1"	16'-12"	18'-1"
+/- 20 psf	120	11'-9"	13'-11"	13'-11"	15'-3"	14'-4"	15'-8"	17'-6"	17'-8"
+/- 20 psf	180	10'-4"	13'-9"	12'-2"	13'-9"	14'-4"	15'-1"	16'-12"	17'-8"
+/- 20 psf	240	9'-4"	12'-6"	11'-1"	12'-6"	13'-4"	13'-9"	15'-5"	16'-6"
+/- 25 psf	120	10'-11"	12'-5"	12'-9"	13'-8"	12'-10"	14'-0"	15'-8"	15'-10"
+/- 25 psf	180	9'-7"	12'-5"	11'-3"	12'-9"	12'-10"	14'-0"	15'-8"	15'-10"
+/- 25 psf	240	8'-8"	11'-7"	10'-3"	11'-7"	12'-5"	12'-9"	14'-4"	15'-3"
+/- 30 psf	120	10'-0"	11'-4"	11'-8"	12'-6"	11'-8"	12'-9"	14'-3"	14'-5"
+/- 30 psf	180	8'-12"	11'-4"	10'-7"	11'-12"	11'-8"	12'-9"	14'-3"	14'-5"
+/- 30 psf	240	8'-2"	10'-11"	9'-8"	10'-11"	11'-8"	11'-12"	13'-6"	14'-5"
+/- 35 psf	120	9'-3"	10'-6"	10'-9"	11'-7"	10'-10"	11'-10"	13'-3"	13'-4"
+/- 35 psf	180	8'-7"	10'-6"	10'-1"	11'-5"	10'-10"	11'-10"	13'-3"	13'-4"
+/- 35 psf	240	7'-9"	10'-4"	9'-2"	10'-4"	10'-10"	11'-5"	12'-9"	13'-4"
+/- 39 psf	120	8'-10"	9'-11"	10'-3"	10'-11"	10'-3"	11'-3"	12'-6"	12'-8"
+/- 39 psf	180	8'-3"	9'-11"	9'-9"	10'-11"	10'-3"	11'-3"	12'-6"	12'-8"
+/- 39 psf	240	7'-6"	9'-11"	8'-10"	9'-12"	10'-3"	10'-12"	12'-4"	12'-8"
+/- 45 psf	120	8'-2"	9'-3"	9'-6"	10'-2"	9'-6"	10'-5"	11'-8"	11'-9"
+/- 45 psf	180	7'-10"	9'-3"	9'-3"	10'-2"	9'-6"	10'-5"	11'-8"	11'-9"
+/- 45 psf	240	7'-2"	9'-3"	8'-5"	9'-6"	9'-6"	10'-5"	11'-8"	11'-9"
+/- 50 psf	120			9'-0"	9'-8"	9'-1"	9'-11"	11'-1"	11'-2"
+/- 50 psf	180			8'-12"	9'-8"	9'-1"	9'-11"	11'-1"	11'-2"
+/- 50 psf	240			8'-2"	9'-2"	9'-1"	9'-11"	11'-1"	11'-2"
+/- 55 psf	120			8'-7"	9'-3"	8'-7"	9'-5"	10'-7"	10'-8"
+/- 55 psf	180			8'-7"	9'-3"	8'-7"	9'-5"	10'-7"	10'-8"
+/- 55 psf	240			7'-11"	8'-11"	8'-7"	9'-5"	10'-7"	10'-8"
+/- 60 psf	120			8'-3"	8'-10"	8'-3"	9'-0"	10'-1"	10'-2"
+/- 60 psf	180			8'-3"	8'-10"	8'-3"	9'-0"	10'-1"	10'-2"
+/- 60 psf	240			7'-8"	8'-8"	8'-3"	9'-0"	10'-1"	10'-2"
+/- 65 psf	120			7'-10"	7'-10"	7'-11"	8'-8"	9'-8"	9'-10"
+/- 65 psf	180			7'-10"	7'-10"	7'-11"	8'-8"	9'-8"	9'-10"
+/- 65 psf	240			7'-6"	7'-6"	7'-11"	8'-8"	9'-8"	9'-10"
+/- 70 psf	120			7'-3"	7'-3"	7'-8"	8'-4"	9'-4"	9'-5"
+/- 70 psf	180			7'-3"	7'-3"	7'-8"	8'-4"	9'-4"	9'-5"
+/- 70 psf	240			7'-3"	7'-3"	7'-8"	8'-4"	9'-4"	9'-5"
+/- 78 psf	120	V ////////		<i>\\\\\\\</i>		7'-3"	7'-11"	8'-10"	8'-11"
+/- 78 psf	180			<i>}////////////////////////////////////</i>		7'-3"	7'-11"	8'-10"	8'-11"
+/- 78 psf	240			X///////		7'-3"	7'-11"	8'-10"	8'-11"
+/- 80 psf	120						<i>\///////</i>	8'-9"	8'-10"
+/- 80 psf	180							8'-9"	8'-10"
+/- 80 psf	240							8'-9"	8'-10"
+/- 85 psf	120			<i>X////////////////////////////////////</i>				8'-6"	8'-7"
+/- 85 psf	180							8'-6"	8'-7"
+/- 85 psf	240			X///////			<i>\///////</i>	8'-6"	8'-7"
+/- 90 psf	120			X///////				8'-3"	8'-4"
+/- 90 psf	180			X///////				8'-3"	8'-4"
+/- 90 psf	240			X///////				8'-3"	8'-4"
+/- 95 psf	120							8'-0"	8'-1"
+/- 95 psf	180						X////////	8'-0"	8'-1"
+/- 95 psf	240			X///////			X////////	8'-0"	8'-1"

^{**} TYPICAL MOUNTING CONNECTION SHOWN, PANELS MAY BE ORIENTED HORIZONTALLY AND/OR VERTICALLY. FOR ALTERNATE MOUNTING CONDITIONS SITE SPECIFIC ENGINEERING IS REQUIRED.

CLEAR SPAN TABLE USE INSTRUCTIONS:

DETERMINE TYPE OF ENCLOSURE TO BE COVERED (OPEN, SCREENED WALLS, OR FULLY ENCLOSED).

VERIFY APPROPRIATE DESIGN LOAD WITH GOVERNING MUNICIPALITY AND BUILDING CODES IN EFFECT FOR THE PROJECT LOCATION USING 2006, 2009 OR 2012 INTERNATIONAL BUILDING CODE (AS APPLICABLE) AS PROVIDED BY SEPARATE ENGINEERING, BY A LICENSED ENGINEER OR REGISTERED ARCHITECT. SEPARATE ENGINEERING MAY BE REQUIRED FOR ALTERNATE DESIGN LOADS

FIND ALLOWABLE COMPOSITE PANEL CLEAR SPAN IN TABLES FOR APPROPRIATE PANEL DEPTH, FACING THICKNESS, AND EPS CORE DENSITY SELECTED.

INDICATES VALUES NOT VALID FOR USE.

DEFLECTION NOTES:

(RECOMMENDED, VERIFY WITH LOCAL JURISDICTION)

USE L/120 FOR ALL MEMBERS SUPPORTING ROOFS OVER AN OPEN OR SCREEN-WALLED ROOM.

USE L/180 FOR ALL MEMBERS SUPPORTING ROOFS WITH A NON-PLASTERED CEILING OVER AN ENCLOSED

USE L/240 FOR ALL MEMBERS SUPPORTING ROOFS WITH A PLASTERED CEILING OVER AN ENCLOSED

OTHER CONSIDERATIONS:

FRONT OVERHANG MAY BE UP TO 3'-0" WITH VALUES LISTED HEREIN. MAXIMUM UNSUPPORTED SIDE OVERHANG IS 25% OF LAST PANEL WIDTH (i.e. 12" MAX FOR 48" PANEL WIDTH).

TO ALUMINUM (6063-T6 MIN. ALLOY): PROVIDE 11/2" MIN. 1/4"Ø SAE GRADE 5 SMS WITH 11/4"Ø MIN. WASHERS, SCREWS SHALL BE 1" LONGER SPACING BETWEEN ANCHORS IN EACH THAN PANEL THICKNESS WITH 11/2" MIN. PAIR, TYP. EDGE DISTANCE, SPACED 1" FROM ENDS WITH THE FOLLOWING BALANCE: TO 0.090" MIN. THICK ALUMINUM: 4" **EXISTING** O.C. MAX HOST STRUCTURE 0.024" OR 0.030" BY OTHERS, TYP. MIN. ALUM OR 26ga 4' MAX WIDTH PER PANEL STEEL TOP & BOTTOM INTERLOCKING PANEL **FACINGS** DEPTH (1/4" PER FOOT MIN SLOPE) 0 0 0 OPTIONAL EPS CORE (1.0 PCF) 0 0 0 **GUTTER** OR DRIP C-CHANNEL, MIN 0.045" CAP THICK (6063-T6 ALUM) OVERHANG — CLEAR SPAN (L) INSIDE TO INSIDE $-#8x\%_6$ " SAE GRADE 5 SMS AT 6" O.C. MIN. 36" MAX O/H AT FRONT & 25% OF LAST PANEL TOP & BOTTOM, TYP. WIDTH AT SIDES (UP TO 12" MAX O/H AT SIDES) ROOF PANEL/SPAN DETAIL

TO WOOD: 1/4"Ø LAG SCREWS WITH 11/2" MIN. THREAD PENETRATION TO **EXISTING WOOD HOST** (G=0.55 OR BETTER), 3/4" MIN. EDGE DISTANCÉ, SPACED AT 6" O.C. MAX. (STAGGERED)

TO CONCRETE: 1/4"Ø ITW SS TAPCONS WITH 13/4" MIN. EMBED. TO EXISTING 3192PSI NON-CRACKED CONCRETE HOST, 21/2" MIN. EDGE DISTANCE, SPACED AT 6" O.C. MAX (STAGGERED).

N.T.S.

TO HOLLOW BLOCK*: 1/4"Ø ITW SS TAPCONS WITH 11/4" MIN. EMBED. TO EXISTING HOLLOW BLOCK, 2½" MIN. EDGE DISTANCE, SPACED AT 6" O.C. MAX. (STAGGERED)

VERT SECTION

* GOOD WORKMANSHIP PRACTICES SHOULD BE FOLLOWED TO AVOID **BLOWOUT OF BACK SIDE** OF HOLLOW BLOCK.

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ACH, FL Fax: (954) FRANK L. BENNARD 160 SW 12th AVENUE DEERFIELD BEACH, F PH: (954) 354-0660 FAX: (954

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TO WOOD (G=0.55 OR BETTER): $\frac{1}{4}$ "Ø LAG SCREWS WITH $\frac{1}{2}$ "Ø MIN. WASHERS, $\frac{1}{2}$ " MIN. THREAD PENETRATION TO EXISTING

WOOD MEMBER, 3/4" MIN. EDGE DIST, SPACED 2" FROM EACH END AND 4" O.C.

PH: (813) 855-2627

S FOAM CORE ROOF PANELS - METAL SK
INTERNATIONAL BUILDING CODE
TEXAS DEPARTMENT OF INSURANCE BUILDING
50 BURBANK ROAD
NLDSMAR, FL 34677 350 BURBANK FOR SOLDSMAR, FL 3

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